

GENERAL NOTES

A. GENERAL:

1. ALL CONSTRUCTION SHALL CONFORM TO 2014 CITY OF LOS ANGELES BUILDING CODE UNLESS MORE STRINGENT REQUIREMENTS ARE REQUIRED IN THE PLANS AND SPECIFICATIONS.
2. THE CONTRACTOR SHALL VERIFY IN THE FIELD ALL CONDITIONS, ELEVATIONS AND DIMENSIONS BEFORE STARTING WORK. THE ARCHITECT AND STRUCTURAL ENGINEER SHALL BE NOTIFIED IMMEDIATELY IF ANY DISCREPANCIES ARE FOUND.
3. THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY DISCREPANCIES OR OMISSIONS NOTED ON THE DRAWINGS OR IN THE SPECIFICATIONS OR OF ANY VARIATIONS NEEDED IN ORDER TO CONFORM TO CODES, RULES AND REGULATIONS. UPON RECEIPT OF SUCH INFORMATION, THE ENGINEER WILL SEND WRITTEN INSTRUCTIONS TO ALL CONCERNED AND WORK SHALL BE PERFORMED IN A MANNER AS DIRECTED BY THE ENGINEER. ANY SUCH DISCREPANCY, OMISSION, OR VARIATION NOT REPORTED, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
4. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE TEMPORARY BRACES, SHORES AND GUYS, WHEREVER NECESSARY TO SUPPORT ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED DURING CONSTRUCTION, INCLUDING ERECTION EQUIPMENT AND ITS OPERATION. THIS TEMPORARY SUPPORT SYSTEM SHALL HOLD ALL ELEMENTS AND MEMBERS IN THEIR FINAL POSITION UNTIL TOTALLY AND FINALLY CONNECTED TO THE PERMANENT BRACING ELEMENTS.
5. CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH THE CALIFORNIA CONSTRUCTION SAFETY ORDERS.
6. REVIEW OF SHOP DRAWINGS BY STRUCTURAL ENGINEER SHALL NOT BE CONSTRUED AS ACCEPTING RESPONSIBILITY FOR SAFE CONSTRUCTION PRACTICES.
7. SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT AND ARE SUPERSEDED BY THE STRUCTURAL DRAWING. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IF ANY DISCREPANCIES ARE FOUND.
8. THE TYPICAL NOTES AND DETAILS SHALL APPLY IN ALL CASES UNLESS SPECIFIC DETAILS OCCUR ELSEWHERE. WHERE NO DETAIL IS SHOWN, CONSTRUCTION SHALL BE AS FOR SIMILAR WORK.
9. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF OPENINGS OR SUPPORTS FOR THEIR RESPECTIVE ITEMS. NOTIFY ARCHITECT, PRIOR TO CONSTRUCTION, OF ANY INTERFERENCE OR INCOMPATIBILITY.
10. THE DEPUTY INSPECTOR SHALL BE REGISTERED WITH THE CITY OR COUNTY OF LOS ANGELES OR ICBO AND PAID BY THE OWNER OR CONTRACTOR.
11. CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A WIND OR SEISMIC FORCE RESISTING SYSTEM / COMPONENT LISTED IN THE 'STATEMENT OF SPECIAL INSPECTION' SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE LABS INSPECTORS AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON SUCH SYSTEM OR COMPONENT PER SEC 1706.01.
12. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS, INCLUDING NAILING, BOLTING, ANCHORING, AND OTHER FASTENING TO COMPONENTS OF THE SEISMIC FORCE RESISTING SYSTEM. SPECIAL INSPECTION BY A DEPUTY INSPECTOR IS REQUIRED WHERE THE PERMITTING AGENCY OF THE SHEATHING IS MORE THAN 4 INCHES ON CENTER.
13. WHERE SPECIAL INSPECTION OR TESTING IS REQUIRED, THE REGISTERED DESIGN PROFESSIONAL CHARGE SHALL INCLUDE A STATEMENT OF SPECIAL INSPECTIONS.
14. A SUPPLEMENTAL PLAN CHECK AND PERMIT WITH THE APPLICABLE FEES ARE REQUIRED FOR DEFERRED ITEMS. THE ENGINEER OF RECORD SHALL REVIEW THE DESIGN OF THE DEFERRED ITEMS AND VERIFY CONFORMANCE WITH THE INTENT OF THE ORIGINAL DESIGN PREFABRICATED STAIRS, HANDRAILS.
15. CONTINUOUS SPECIAL INSPECTION BY A REGISTERED DEPUTY INSPECTOR IS REQUIRED FOR FIELD WELDING, CONCRETE STRENGTH $f'_{c} > 2500$ PSI, HIGH STRENGTH BOLTING, SPRAYED-ON FIREPROOFING, ENGINEERED MASONRY, HIGH-LIFT GROUTING, PRE-STRESSED CONCRETE, HIGH LOAD DIAPHRAGMS AND SPECIAL MOMENT-RESISTING CONCRETE FRAMES.
16. A COPY OF THE LOS ANGELES RESEARCH REPORT AND/OR CONDITIONS OF LISTING SHALL BE MADE AVAILABLE AT THE JOB SITE.

B. CONCRETE:

1. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH IN 28 DAYS EQUAL TO:
 - a) 3000 PSI FOR COLUMN PAD AND WALL FOOTINGS.
 - b) 2500 PSI FOR SLABS ON GRADE
 - c) 3000 PSI FOR GRADE BEAM AND PILE
 - d) 3000 PSI FOR CONC. WALL
 - e) 3000 PSI FOR CONC. DECK ON GRADE
2. CONTINUOUS DEPUTY INSPECTION REQUIRED FOR ALL CONCRETE STRENGTHS EXCEEDING 2500 PSI.
3. ALL CONCRETE SHALL BE NORMAL WEIGHT (145 PCF) UNLESS NOTED AS LIGHT-WEIGHT CONCRETE.
4. PORTLAND CEMENT: ASTM C150.
5. EXPOSED CONCRETE FORMING AND FINISH SHALL BE AS NOTED ON ARCHITECTURAL DRAWINGS. SEE ARCHITECT DRAWINGS FOR DETAILS.
6. PROJECTING CORNERS OF BEAMS, COLUMNS, ETC. SHALL BE FORMED WITH 3/4" CHAMFER UNLESS OTHERWISE DETAILED.

C. REINFORCING STEEL FOR CONCRETE:

1. REINFORCING BARS: ASTM A615 GRADE 60, GRADE 40 FOR #3 BARS. ALL REINFORCING STEEL THAT IS TO BE WELDED SHALL CONFORM TO ASTM A706, UNLESS OTHERWISE NOTED ON PLANS.
- 1A. REINFORCING BARS IN USED CONCRETE SHEAR WALL AND GRADE BEAM: ASTM A706 OR A615 GRADE 60 SHALL BE PERMITTED IN THESE MEMBERS IF:
 - a) THE ACTUAL YIELD STRENGTH BASED ON MIL TEST DOES NOT EXCEED YIELD STRENGTH BY MORE THAN 18,000 PSI.
 - b) THE RATIO OF THE ACTUAL TENSILE STRENGTH TO THE ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25
2. FIELD WELDING OF REINFORCING STEEL SHALL BE DONE BY CITY CERTIFIED WELDERS. CONTINUOUS DEPUTY INSPECTION REQUIRED.
3. WELDED WIRE FABRIC: ASTM A185
4. WELDING ELECTRODES: ASTM A233, CLASS E90, LOW HYDROGEN.
5. MINIMUM SPLICE LENGTHS, UNLESS DETAILED OTHERWISE.
 - a) CONCRETE-36 DIAMETERS.
 - b) MASONRY-40 DIAMETERS.
 - c) WELDED WIRE FABRIC SHALL BE SPLICED WITH A MINIMUM LAP OF 12 INCHES.
6. MINIMUM CLEARANCE BETWEEN REINFORCING AND FACE OF CONCRETE SHALL BE AS FOLLOWS (UNLESS SHOWN OTHERWISE):
 - a) CONCRETE BELOW GRADE (CAST AGAINST SOIL)=3"
 - b) CONCRETE BELOW GRADE (FORMED)=2"
 - c) CONCRETE WALLS EXPOSED TO WEATHER: NUMBER 5 BARS AND SMALLER=1 1/2". NUMBER 6 BARS AND LARGER=2"
7. MINIMUM CLEARANCE BETWEEN REBAR AND MASONRY SHALL BE 3/4".

D. MASONRY:

1. CONCRETE BLOCK MASONRY UNITS: ASTM C90, GRADE N, $f_m = 1500$ PSI, MEDIUM WEIGHT. MASONRY UNIT COLOR AND FACE TEXTURED AS NOTED ON ARCHITECTURAL DRAWINGS. MASONRY UNIT SHALL BE CLIMATIZED TO SITE.
2. CONTINUOUS INSULATION IS REQUIRED, UNLESS NOTED OTHERWISE, FOR:
 - a) PREPARATION AND FORMING OF WALL PRIMERS.
 - b) SAMPLING AND PLACING OF MASONRY UNITS.
 - c) PLACEMENT OF REINFORCEMENT.
 - d) GROUT SPACE IMMEDIATELY PRIOR TO CLOSING OF CLEANOUTS AND DURING ALL GROUTING OPERATIONS.
3. ALL MORTAR AND GROUT SHALL HAVE A MINIMUM 28 DAYS COMPRESSIVE STRENGTH EQUAL TO 1800 PSI AND 2000 PSI, RESPECTIVELY.
4. PORTLAND CEMENT -ASTM C150, TYPE II, LOW ALKALI.
5. MORTAR MIX-1:3 WITH 1/4 PART LIME PUTTY, TYPE S.
6. GROUT MIX-1:3 WITH 2 PARTS PEA GRAVEL.
7. GROUT ALL CELLS, UNLESS OTHERWISE NOTED.
8. LOCATE REBARS ABOUT CENTERLINE OF MASONRY WALL UNLESS DETAILED OTHERWISE.
9. ALL WALLS SHALL BE CONSTRUCTED USING 1/2 RUNNING BOND BETWEEN MASONRY UNITS, UNLESS OTHERWISE NOTED.

E. LUMBER:

1. ALL LUMBER SHALL BE DOUGLAS FIR-LARCH, S4S, UNLESS NOTED OTHERWISE, AND IT SHALL BE MANUFACTURED GRADED, AND BEAR THE GRADE MARK OF WCLB STANDARD GRADING RULES 16 OR WPMA GRADING RULES; MOISTURE CONTENT AT TIME OF INSTALLATION SHALL NOT BE OVER 19% OR LESS THAN 7%.
2. ALL FRAMING LUMBER SHALL BE VISUALLY GRADED DIMENSION LUMBER UNDO.
 - a) JOISTS AND RAFTERS #2 GRADE OR BETTER (U.N.O.)
 - b) BEAMS OR 4X6 POST OR LARGER #1 GRADE OR BETTER (U.N.O.)
 - c) STUDS #2 GRADE OR BETTER (U.N.O.)
3. PLYWOOD SHEATHING SHALL BE DOUGLAS-FIR CONFORMING TO PS-1-95, GROUP 1 OR 2 OR APA PERFORMANCE RATED PANELS (I.E. PLYWOOD, OSB, OR COMPOSITE BOARD). ALL PLYWOOD/OSB TO BE 'EXTERIOR' TYPE, GRADE-MARKED BY THE AMERICAN PLYWOOD ASSOCIATION, GRADE CDX UNLESS NOTED OTHERWISE.
4. NAILING OF ROOF AND FLOOR SHEATHING SHALL BE INSPECTED AND APPROVED BY THE BUILDING DEPARTMENT BEFORE COVERING. FACE GRADE OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORTS. FLOOR SHALL HAVE TONGUE AND GROOVE OR BLOCKED PANEL EDGES. PLYWOOD SPANS SHALL CONFORM WITH TABLE 2304.7
- 4a. ALL DIAPHRAGM AND SHEAR WALL NAILING SHALL UTILIZE COMMON NAILS OR GALVANIZED BOX
5. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED IN ACCORDANCE WITH AWPA STANDARD C1 AND AWPA STANDARD LP-2, EACH PIECE OF LUMBER SHALL BE THE MARK OF AN APPROVED TESTING AGENCY THE MAXIMUM MOISTURE CONTENT IS 14% WHENEVER IT IS NECESSARY TO CUT, NOTCH, BORE OR SPLICE TREATED LUMBER, THOROUGHLY PAINT NEWLY CUT SURFACE WITH SAME PRESERVATIVE USED IN THE ORIGINAL TREATMENT OF THE LUMBER.
- 5a. FASTENERS IN PRESERVATIVE-TREATED & FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT DIPPED ZINC-COAT GALVANIZED STEEL.
6. MACHINE APPLIED NAILING SHALL NOT BE USED UNLESS SATISFACTORY INSTALLATION IS DEMONSTRATED ON THE JOB AND ITS USE IS APPROVED BY THE GOVERNING CODE INSPECTORS. APPROVAL IS SUBJECT TO CONTINUED SATISFACTORY PERFORMANCE.
7. BOLT HOLES IN WOOD SHALL BE 1/32" TO 1/16" OVERSIZE. HOLES OVER 1/16" LARGER SHALL REQUIRE REPLACEMENT OF LUMBER PIECE. INSPECTOR TO VERIFY.
8. ALL BOLTS BEARING ON WOOD SHALL HAVE WASHERS.
9. APPROVED PLATE WASHERS TO BE USED FOR PLYWOOD SHEAR WALL SILL PLATE ANCHOR BOLTS AND HOLLOW CONECTOR BOLTS AT SHEAR WALL.

MINIMUM SIZE FOR SQUARE PLATE WASHERS

BOLT SIZE	PLATE WASHERS
1/2" ϕ	1/4" x 3" x 3"
5/8" ϕ	1/4" x 3" x 3"
3/4" ϕ	5/16" x 3" x 3"
7/8" ϕ	3/8" x 3" x 3"
1" ϕ	3/8" x 3 1/2" x 3 1/2"

10. ANCHORAGE OF WOOD TO CONCRETE, MASONRY OR STEEL:

- a) ALL SILL PLATE BOLTS SHALL BE 5/8" DIAMETER, MINIMUM EMBEDMENT 9", LOCATED AT 4'-0" ON CENTER UNLESS OTHERWISE NOTED.
- b) SECURE WOOD LEDGER WITH 3/4" DIAMETER BOLTS AT 4'-0" ON CENTER UNLESS OTHERWISE NOTED.
- c) THE END STUD OF ALL PARTITIONS ABUTTING MASONRY OR CONCRETE SHALL BE ANCHORED WITH 1/2" DIAMETER BOLTS AT TOP, BOTTOM AND 4'-0" ON CENTER.
- d) ALL WOOD MEMBERS SHALL HAVE AN ANCHOR BOLT BETWEEN 6 TO 9 INCHES FROM EACH END. LENGTH OF ANCHOR BOLTS SHALL BE SUFFICIENT TO GRIP FULL NUT AND BE ANCHORED AT LEAST 6" PLUS HOOK INTO MASONRY OR CONCRETE.
- e) USE 1/2" DIAMETER STUD BOLTS AT 2'-0" ON CENTER TO STRUCTURAL STEEL UNLESS OTHERWISE NOTED.
- f) INTERIOR NON-BEARING WALL SILL PLATES MAY BE ANCHORED TO CONCRETE WITH 7/32" DIAMETER POWER DRIVEN PINS AT 30" ON CENTER UNLESS DETAILED OTHERWISE.

11. CONNECTOR DESIGNATIONS REFER TO STRONG - TIE CONNECTORS BY SIMPSON COMPANY, BREA, CALIFORNIA, UNLESS NOTED OTHERWISE. SPECIAL NAILS OF THE SAME DIAMETER AND PENETRATING INTO THE SUPPORT ONE HALF OF THE LENGTH CALLED FOR, MAY BE USED WITH SIMPSON METAL CONNECTORS, A35-LARR 25716, STRAP-LARR 25713, FACE MOUNT HANGER-LARR 25807, TOP FLANGE HANGER-LARR 25800, TJI HANGER-LARR 25803, COL. CAP-LARR 25714.
12. NAIL ALL 2X DOUBLE JOIST WITH 16d NAILS @ 9" ON CENTER.

13. PROVIDE DOUBLE JOIST UNDER ALL PARALLEL PARTITIONS.

14. PROVIDE 2" FIRE BLOCKING IN STUD WALLS AT MAXIMUM VERTICAL SPACING PERMITTED BY GOVERNING CODE AND AT ALL CEILING LINES.

15. FOR SIZE AND LOCATION OF ROOF, FLOOR AND WALL OPENING, SEE ARCHITECTURAL MECHANICAL AND ELECTRICAL DRAWINGS. FRAME ALL FOUR SIDES OF OPENINGS WITH ADEQUATE MEMBERS AND CONNECTORS.

16. NON-BEARING WOOD STUD WALLS SHALL BE BRACED ALONG THEIR LENGTH AT LEAST ONCE IN EVERY 25 FEET OF LENGTH. THE BRACING SHALL BE A 1"x4" CONTINUOUS LET-IN BOARD NAILED TO TOP AND BOTTOM PLATES OR SIMPSON 16 GAUGE WB STRAPS IN EACH DIRECTION (X-BRACING). RUN BRACE DIAGONALLY FROM BOTTOM PLATE TO UPPER TOP PLATE AT AN ANGLE SUFFICIENT TO INCLUDE FOUR NORMAL STUD SPACES FOR EVERY 12 FOOT INCREMENT OF HEIGHT.

17. ALL RAFTERS AND FLOOR JOISTS WITH DEPTH TO THICKNESS RATIO OF 6 OR GREATER SHALL BE SOLID BLOCKED @ 8'-0" O.C.

18. STRUCTURAL WOOD PANEL SHEAR WALLS SHALL BE COVERED WITH MIN. 2 LAYERS 15# FELT UNDERLAYMENT PRIOR TO PLACING FINISH MATERIAL.

19. ALL HORIZONTAL JOINTS OCCURRING IN BRACED WALL PANELS SHALL OCCUR OVER BLOCKING EQUAL IN SIZE TO THE STUDDING.

20. LAG BOLTS: PROVIDE LEAD HOLE 70% OF THREADED SHANK DIAMETER, AND FULL DIAMETER FOR SMOOTH SHANK PORTION. SOAP, PARAFFIN OR OTHER APPROVED LUBRICANT SHALL BE USED ON THREADS. INSTALLATION SHALL BE BY SCREWING NOT HAMMERING. CARE SHALL BE TAKEN TO AVOID OVER TORQUING BOLT. (91.2337)

21. ALL HORIZONTAL JOINTS OCCURRING IN BRACED WALL PANELS SHALL OCCUR OVER BLOCKING EQUAL IN SIZE TO THE STUDDING.

22. ALL NAILING SHALL BE IN ACCORDANCE WITH CALIFORNIA BUILDING CODE REQUIREMENTS UNLESS NOTED OTHERWISE ON THE DRAWINGS. USE COMMON WIRE NAILS.

NAILING SCHEDULE

1. JOIST TO SILL OR GIRDER, TOENAIL ----- 3-8d
2. BRIDGING TO JOIST, TOENAIL EACH END ----- 2-8d
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL ----- 2-8d
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL ----- 3-8d
5. 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL ----- 2-16d
6. SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL ----- 16d @ 16" O.C.
7. SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANELS ----- 3-16d per 16"
8. STUD TO STUD, END NAIL ----- 2-16d
9. STUD TO SOLE PLATE ----- 4-8d, toenail or
10. DOUBLE STUDS, FACE NAIL ----- 16d @ 24" O.C.
11. DOUBLED TOP PLATES, TYPICAL FACE NAIL ----- 16d @ 16" O.C.
12. DOUBLED TOP PLATES, LAP SPLICE ----- 8-16d
13. BLOCKING BETWEEN JOISTS ----- 3-8d
14. OR RAFTERS TO TOP PLATE, TOENAIL ----- 8d @ 6" O.C.
15. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL ----- 2-16d
16. CONTINUOUS HEADER, TWO PIECES ----- 16d @ 16" O.C. along each edge
17. CEILING JOISTS TO PLATE, TOENAIL ----- 3-8d
18. CONTINUOUS HEADER TO STUD, TOENAIL ----- 4-8d
19. CEILING JOISTS, LAP OVER PARTITIONS, FACE NAIL ----- 3-16d
20. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL ----- 3-16d
21. RAFTER TO PLATE, TOENAIL ----- 3-8d
22. 1" BRACE TO EACH STUD AND PLATE, FACE NAIL ----- 2-8d
23. 1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL ----- 2-8d
24. WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL ----- 3-8d
25. BUILT-UP CORNER STUDS ----- 16d @ 24" O.C.
26. 2" PLANKS ----- 2-16d at each bearing

23. GLUE LAMINATED MEMBERS:

- a) MANUFACTURED IN ACCORDANCE WITH LATEST AITC 117 SPECIFICATIONS.
- b) DOUGLAS FIR, NORTH REGION LUMBER.
- c) COMBINATION 24F-V4 STRESS GRADE ($F_b = 2400$ PSI).
- d) COMBINATION 24F-V8 STRESS GRADE ($F_b = 2400$ PSI) AT BEAMS WITH CANTILEVER.
- e) AITC APPEARANCE GRADE - SEE ARCH. DRAWINGS OR SPECIFICATIONS.
- f) MOISTURE CONTENT - 15 PERCENT MAXIMUM
- g) INSPECTION - AITC CERTIFICATE.
- h) ALL GLU-LAMINATED MEMBERS FABRICATION SHALL BE L.A. CITY BLDG. DEPT. CITY LICENSED FABRICATOR.
- i) GLB MUST BE FABRICATED IN L.A. CITY BLDG. DEPT. LICENSED SHOP. IDENTIFY GRADE SYMBOL AND LAMINATION SPECIES PER T 5-A, 2005 NDS SUPP.

24. PARALLAM PSL & MICOR-LAM LVL:

- a) PARALLAM PSL SPECIFIED IN THESE PLANS AS PSL ARE TO BE MANUFACTURED BY TRUSS JOIST MODULUS OF ELASTICITY $E = 2.0 \times 10^6$ PSI
- b) MICORLAM LVL SPECIFIED IN THESE PLANS AS PSL ARE TO BE MANUFACTURED BY TRUSS JOIST MODULUS OF ELASTICITY $E = 1.9 \times 10^6$ PSI
- c) PARALLAM PSL AND MICORLAM LVL ARE NON-TREADED, DRY SERVICE CONDITION ONLY.
- d) BRIDGING, BLOCKING OR FRAMING AS SHOWN ON PLAN SHALL BE PROVIDED PER MANUFACTURE REQUIREMENTS. LARR 25202

25. SOLID WEB BUILT-UP JOISTS:

- a) THE TJI JOIST SPECIFIED IN THESE PLANS ARE TO BE MANUFACTURED BY TRUSS JOIST
- b) SOLID WEB JOIST MANUFACTURER SHALL PROVIDE FIELD INSPECTION AND REPLACEMENT FOR ANY DAMAGED, SPLIT OR OTHERWISE STRUCTURALLY UNSOUND SOLID WEB MEMBER.
- c) BRIDGING OR FRAMING AS SHOWN ON PLAN SHALL BE PROVIDED PER MANUFACTURER REQUIREMENTS. LARR 25538

F. STRUCTURAL STEEL:

1. STRUCTURAL STEEL SHALL CONFORM TO:
 - a) STRUCTURAL W SHAPES: A992, GRADE 50
 - b) ALL MOMENT FRAME COLUMN & BEAMS ASTM A992, GRADE 50
 - c) STRUCTURAL C, MC, L, WT, SHAPES AND PLATE: A36
 - d) HSS: ASTM A500, GRADE B.
 - e) PIPE COLUMNS: ASTM A53, GRADE B.
 - f) ELECTRODES: ASTM A233, CLASS E70XX.
 - g) MACHINE BOLTS: ASTM A307.
 - h) HIGH STRENGTH BOLTS: ASTM A325/A490 (PER PLANS AND DETAILS)

2. ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS.

3. SHOP WELDS MUST BE PERFORMED IN A L.A. CITY BLDG. DEPT. LICENSED FABRICATOR'S SHOP
4. ALL WELDING SHALL BE DONE BY WELDERS MUST BE CERTIFIED BY THE L.A. CITY BUILDING DEPARTMENT FOR STRUCTURAL STEEL REINFORCING STEEL, LIGHT GAGE STEEL, CONTINUOUS DEPUTY INSPECTION IS REQUIRED FOR ALL FIELD WELDING AND HIGH STRENGTH BOLTS.

LATERAL FORCE RESISTING SYSTEM FOR THIS BUILDING IS AN INTERMEDIATE MOMENT FRAME

G. SOIL:

1. SOIL REPORT BY BYER GEOTECHNICAL, INC., DATE DECEMBER 22, 2014, FILE NO: BG 22056
2. SOIL REPORTS ARE PART OF STRUCTURAL PLAN
3. TYPE OF SOIL: BEDROCK
4. ALLOWABLE BEARING = 4,000 PSF
5. INSPECTION FOR FOUNDATION EXCAVATION REQUIRED PER SOILS REPORT RECOMMENDATIONS.

H. PROJECT DATA

DESIGN LOADS : ROOF LL= 20 PSF
FLOOR LL= 40 PSF

WIND DESIGN DATA

1. BASIC WIND SPEED = 110 MPH
2. WIND IMPORTANCE FACTOR $I = 1.0$
3. OCCUPANCY CATEGORY II
4. WIND EXPOSURE B
5. INTERNAL PRESSURE COEFFICIENT = ± 0.18
6. COMPONENTS AND CLADDING DESIGN WIND PRESSURE FOR EFF. WIND AREA OF 10 SQ. FT.

ZONE	+ (psf)	-(psf)
1	10	9
2	10	9
3	12	11
4	12	11
5	22	20

EARTHQUAKE DESIGN DATA

1. IMPORTANCE FACTOR, $I = 1.0$
2. $S_S = 2.288$
3. $S_1 = 0.846$
4. SITE CLASS = D
5. $S_0S = 1.524$
6. $SD_1 = 0.846$
7. BASIC SEISMIC FORCE RESISTING SYSTEMS LIGHT-FRAMED WALLS
8. DESIGN BASE SHEAR
9. $E-W = 4.9$ psf
10. $N-S = 0.164$
11. $E-W = 0.164$
12. $N-S = 0.164$
13. RESPONSE MODIFICATION FACTOR, $R = 6.5$
14. EQUIVALENT LATERAL FORCE PROCEDURE
15. REDUNDANCY FACTOR = 1.0
16. SOIL BEARING PRESSURE = 4000 PSF

J. SPECIAL INSPECTION

1. SPECIAL INSPECTION REQUIRED FOR CONC. STRENGTH OVER 2500 PSI
2. SPECIAL INSPECTION REQUIRED FOR FIELD WELDING
3. SPECIAL INSPECTION REQUIRED FOR HIGH STRENGTH BOLTS.
4. SPECIAL INSPECTION REQUIRED FOR UNIDENTIFIED PANEL ANCHOR BOLTS
5. SPECIAL INSPECTION REQUIRED FOR SIMPSON SET-UP
6. SPECIAL INSPECTION REQUIRED FOR FOUNDATION EXCAVATION

Los Angeles Regional Uniform Code Program

Committee I-3: Structural Observation

STRUCTURAL OBSERVATION PROGRAM

AND DESIGNATION OF THE
STRUCTURAL OBSERVER

PROJECT ADDRESS: 1030 N. ANOKA PLACE PERMIT APPL. NO.:

Description of Work : REMODEL & ADDITION

Owner: Architect: Engineer: BERKOZ & ASSOCIATES

STRUCTURAL OBSERVATION (only checked items are required)			
Firm or individual to be responsible for the Structural Observation: Name: BERKOZ & ASSOCIATES Phone: (818) 668-8589 Calif. Registration: SE 2504			
FOUNDATION	WALL	FRAME	DIAPHRAGM
<input checked="" type="checkbox"/> Floating, Stem Walls, Piers	<input type="checkbox"/> Concrete	<input checked="" type="checkbox"/> Steel Moment Frame (Checklist see sheet Sd-5)	<input type="checkbox"/> Concrete
<input type="checkbox"/> Mat Foundation	<input type="checkbox"/> Masonry	<input type="checkbox"/> Steel Brace Frame	<input type="checkbox"/> Masonry
<input checked="" type="checkbox"/> Caisson, Piles, Grade Beams	<input checked="" type="checkbox"/> Wood	<input type="checkbox"/> Concrete Moment Frame	<input checked="" type="checkbox"/> Wood
<input type="checkbox"/> Steep/g/Retain'g Foundation, Hillside Special Anchor	<input type="checkbox"/> Others:	<input type="checkbox"/> Masonry wall frame	<input type="checkbox"/> Others:
<input type="checkbox"/> Others:		<input type="checkbox"/> Others:	

DECLARATION BY OWNER

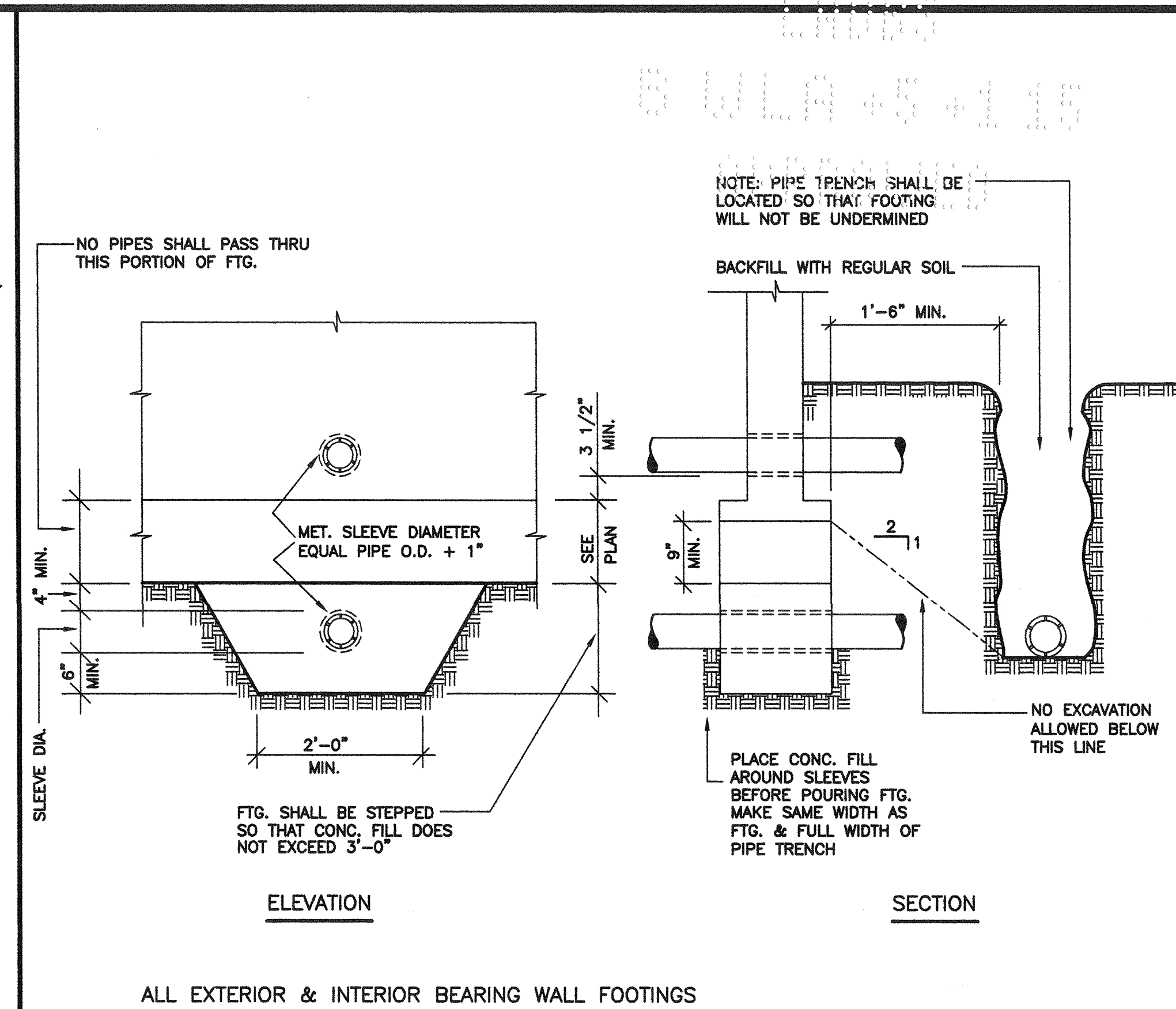
I, the owner of the project, declare that the above listed firm or individual is hired by me to be the Structural Observer.

Signature _____ Date _____

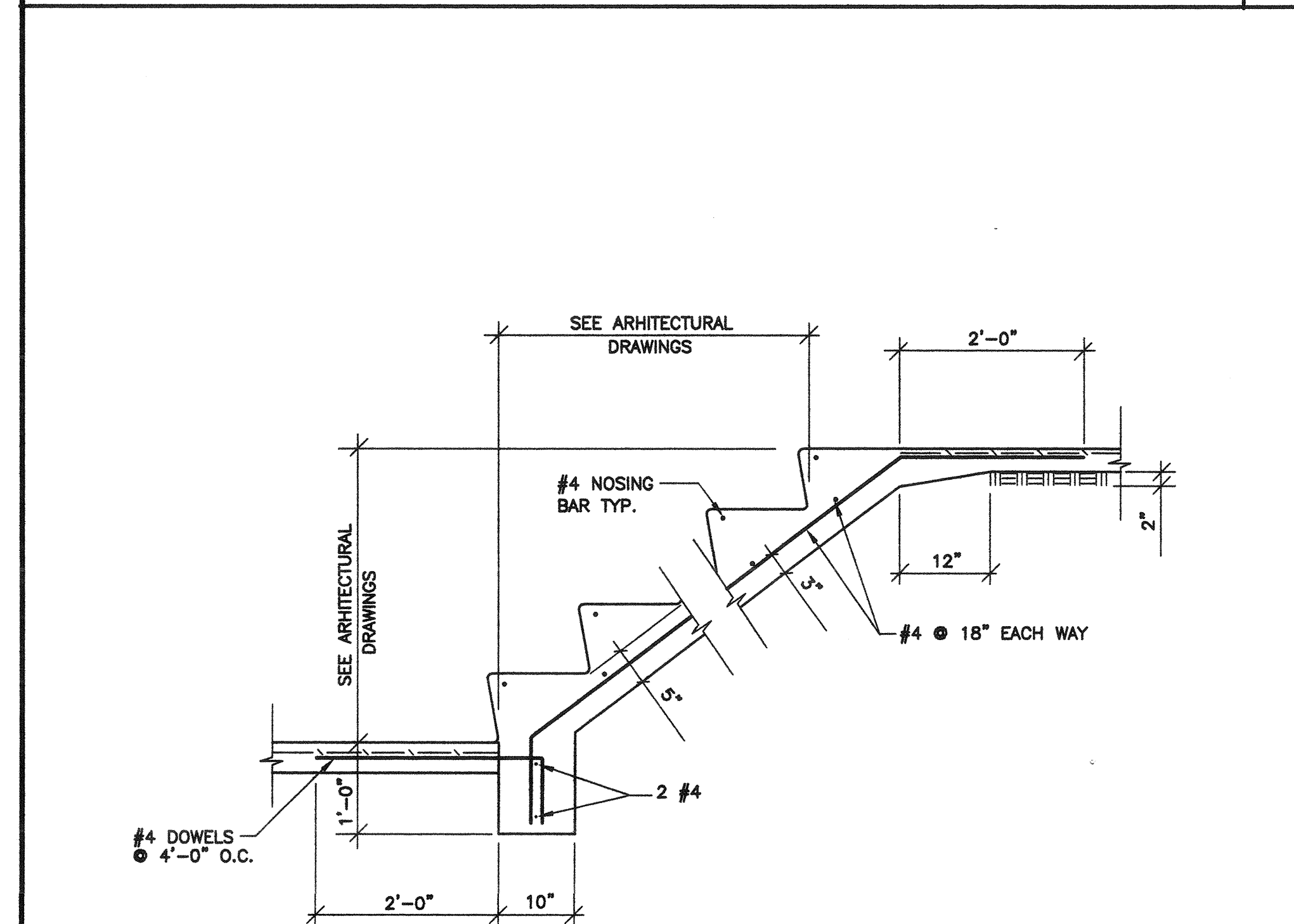
DECLARATION BY ARCHITECT OR ENGINEER OF RECORD

(required if the Structural observer is different from the Architect or Engineer of Record)
I, the Architect or Engineer of the project, declare that the above listed firm or individual is designated by to be responsible for the Structural Observation.

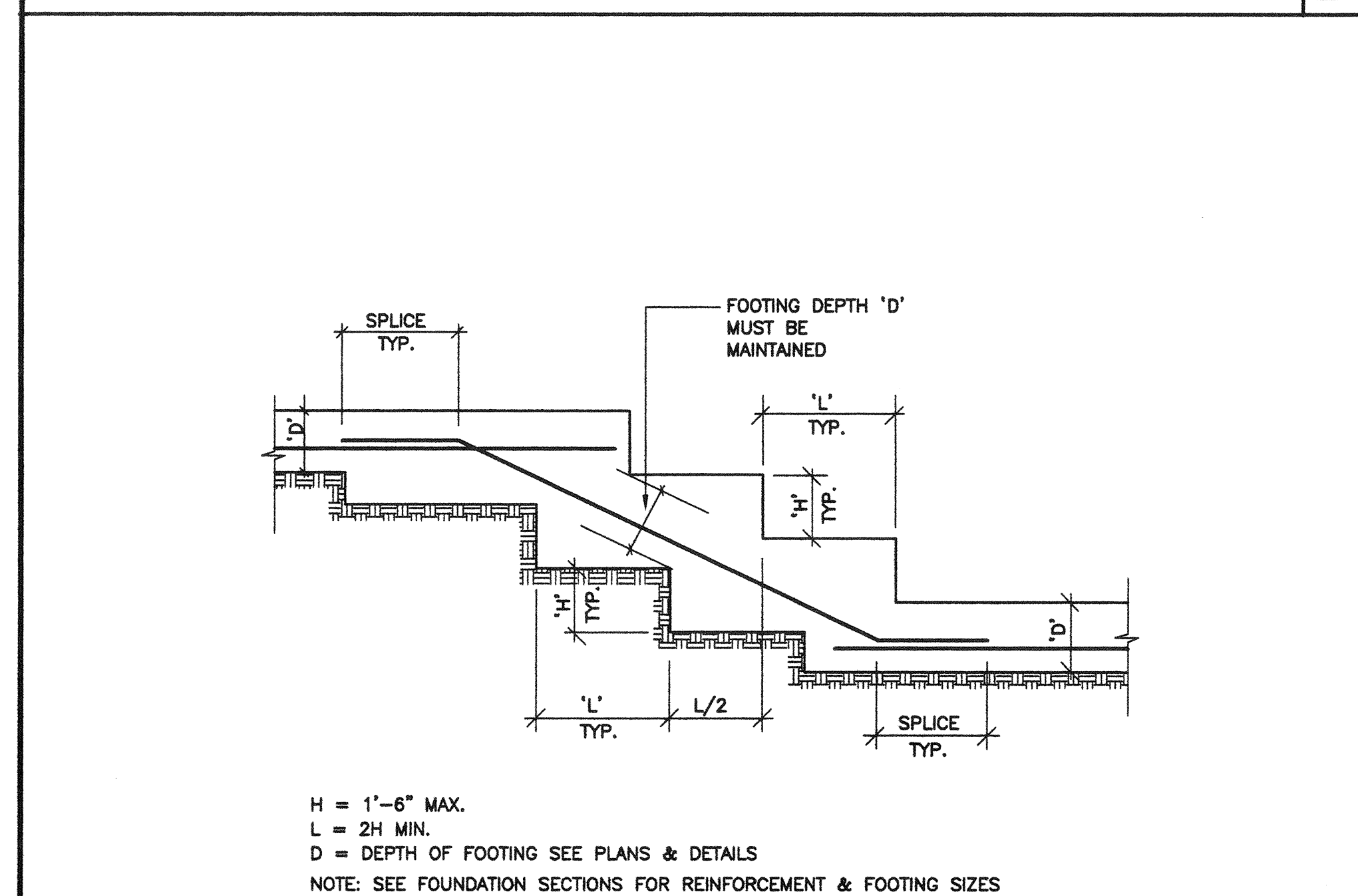
Signature _____ License No. _____ Date _____



PIPE & TRENCH LOCATION DETAIL



TYPICAL STEP



H = 1'-6" MAX.
L = 2H MIN.
D = DEPTH OF FOOTING SEE PLANS & DETAILS
NOTE: SEE FOUNDATION SECTIONS FOR REINFORCEMENT & FOOTING SIZES

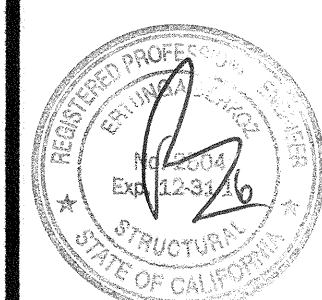
TYPICAL STEP FOOTING

REVISION BY

4-21-2015

REMODEL AND ADDITION
EXISTING RESIDENCE AT
1030 N. ANOKA PL.
PACIFIC PALISADES, CA 90272

BERKOZ & ASSOCIATES, INC.
CONSULTING STRUCTURAL ENGINEERS
5530 CORBIN AVENUE, SUITE 355
TARZANA, CA 91356
(818) 668-8589 FAX: (818) 668-8586



DATE 2-24-2015

JOB #

STRUCTURAL

S-1